ARKANSAS STORAGE CENTER

BRYANT, AR

DRAINAGE REPORT

FOR
City of Bryant, Saline County, AR

December 2023

Owner & Developer: STUART FINLEY
Address: P.O Box 10, Bryant, AR. 72089

By:

HOPE
CONSULTING
ENGINEERS • SURVEYORS

PROJECT TITLE

ARKANSAS STORAGE CENTER

PROJECT PROPERTY OWNER

STUART FINLEY

PROJECT LOCATION

25300 I-30 North, Bryant, AR

PROJECT DESCRIPTION

The proposed self-storage facility development is located on High-way I-30 in the city of Bryant, Arkansas. The total development area is 24.31 acres.

DRAINAGE ANALYSIS

On Site Drainage- Rational method was used to determine the existing and proposed flows from proposed site. Detailed drainage calculations considering the future expected development have been conducted. Summary of the calculations are below:

- Pre-development area: 28.91 acres.
- Post-development area: 28.91 acres.
- Pre-development runoff coefficient: 0.47.
- Post-development runoff coefficient: 0.88.
- Time of Concentration for Pre-development Area: 16.05 min
- Time of Concentration for Post-development Area: 10.58 min
- One 12" RCP with 0.5% slope is proposed for outflow culvert with an elevation of 353.5'

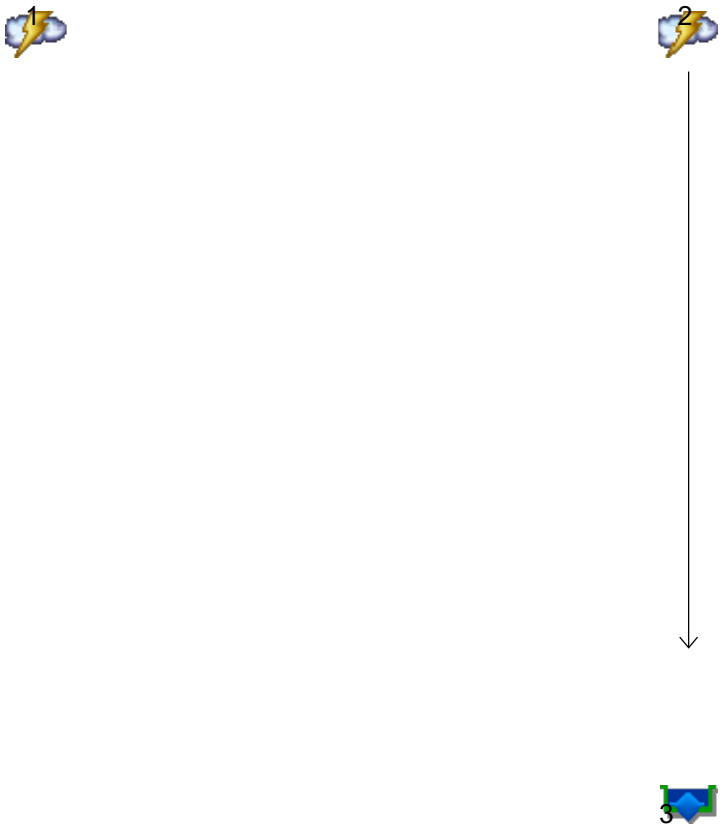
Peak flows for Pre and post development phase of onsite area have been tabulated below-

	Pre-Development	Post-Development without Detention	Post-Development with Detention
	Peak Flow (cfs)	Peak Flow (cfs)	Peak Flow (cfs)
2-Year	53.08	116.86	1.831
5-Year	58.66	130.27	3.033
10-Year	69.15	149.65	5.315
25-Year	79.33	176.93	8.288
50-Year	90.45	194.22	11.98
100-Year	96.16	205.89	13.95
TOC	**16.05 min**	**10.58 min**	

CONCLUSION

The onsite drainage calculation for pre and post condition has been provided.

Watershed Model Schematic



Legend

Hyd.	Origin	Description
1	Rational	Pre-development
2	Rational	Post-Development
3	Reservoir	Pond

Hydrograph Return Period Recap

Hyd. No.	Hydrograph type (origin)	Inflow hyd(s)	Peak Outflow (cfs)								Hydrograph Description
			1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	
1	Rational	------	-------	53.08	-------	58.66	69.15	79.33	90.45	96.16	Pre-development
2	Rational	------	-------	116.86	-------	130.27	149.65	170.93	194.22	205.89	Post-Development
3	Reservoir	2	-------	1.831	-------	3.033	5.315	8.288	11.98	13.95	Pond

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2024

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	53.08	1	16	50,961	------	------	------	Pre-development
2	Rational	116.86	1	11	77,130	------	------	------	Post-Development
3	Reservoir	1.831	1	22	69,981	2	354.52	75,979	Pond

Hydrograph Report

Hyd. No. 1

Pre-development

Hydrograph type	=	Rational	Peak discharge	=	53.08 cfs
Storm frequency	=	2 yrs	Time to peak	=	0.27 hrs
Time interval	=	1 min	Hyd. volume	=	50,961 cuft
Drainage area	=	28.910 ac	Runoff coeff.	=	0.47
Intensity	=	3.907 in/hr	Tc by User	=	16.00 min
IDF Curve	=	Bryant 50.IDF	Asc/Rec limb fact	=	1/1



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2024

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	58.66	1	16	56,310	------	------	------	Pre-development
2	Rational	130.27	1	11	85,979	------	------	------	Post-Development
3	Reservoir	3.033	1	22	78,671	2	354.64	84,478	Pond

Hydrograph Report

Hyd. No. 1

Pre-development

Hydrograph type	=	Rational	Peak discharge	=	58.66 cfs
Storm frequency	=	5 yrs	Time to peak	=	0.27 hrs
Time interval	=	1 min	Hyd. volume	=	56,310 cuft
Drainage area	=	28.910 ac	Runoff coeff.	=	0.47
Intensity	=	4.317 in/hr	Tc by User	=	16.00 min
IDF Curve	=	Bryant 50.IDF	Asc/Rec limb fact	=	1/1



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2024

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	69.15	1	16	66,385	------	------	------	Pre-development
2	Rational	149.65	1	11	98,770	------	------	------	Post-Development
3	Reservoir	5.315	1	22	91,328	2	354.79	96,410	Pond

Hydrograph Report

Hyd. No. 1

Pre-development

Hydrograph type	=	Rational	Peak discharge	=	69.15 cfs
Storm frequency	=	10 yrs	Time to peak	=	0.27 hrs
Time interval	=	1 min	Hyd. volume	=	66,385 cuft
Drainage area	=	28.910 ac	Runoff coeff.	=	0.47
Intensity	=	5.089 in/hr	Tc by User	=	16.00 min
IDF Curve	=	Bryant 50.IDF	Asc/Rec limb fact	=	1/1



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2024

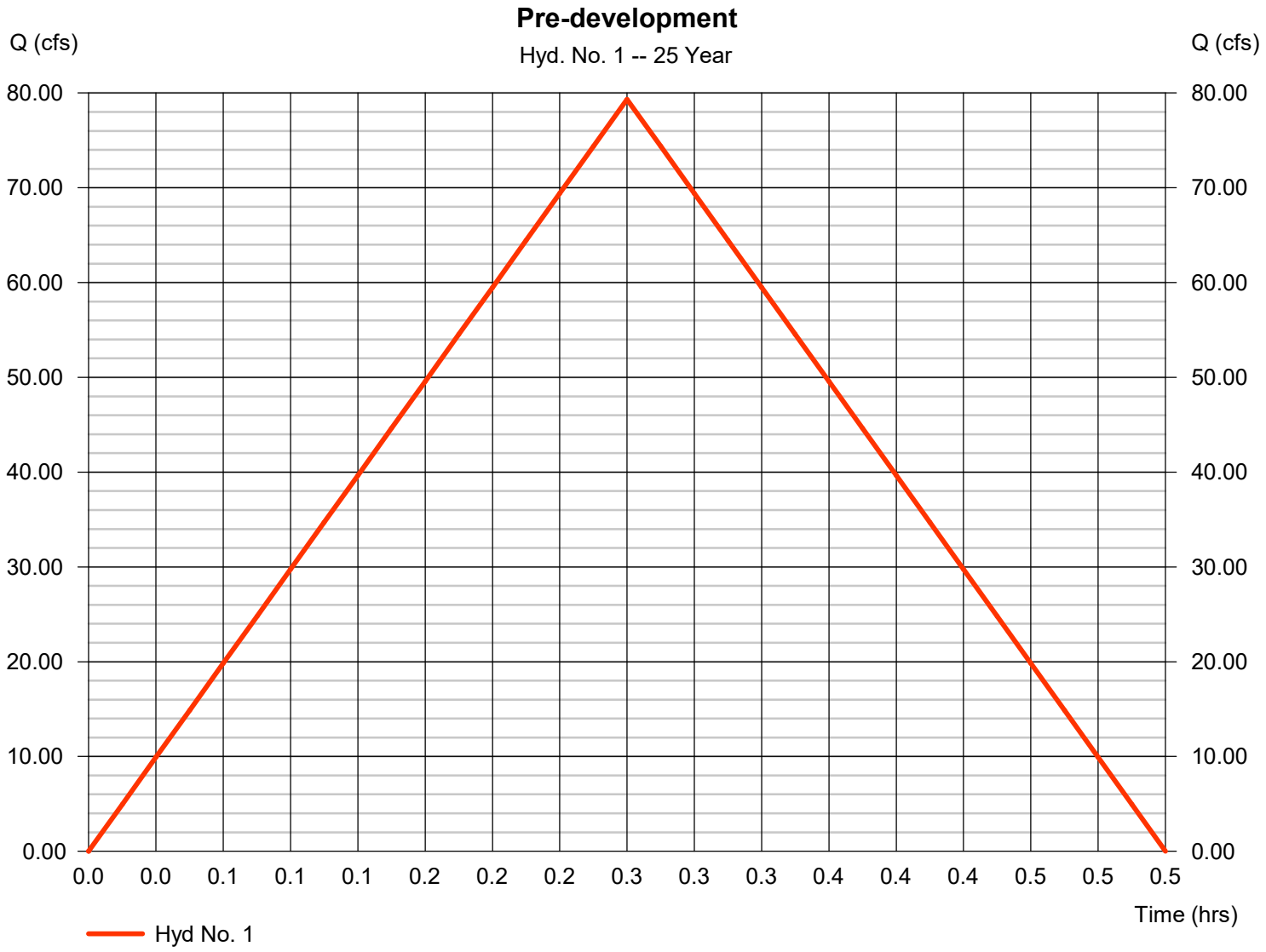
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	79.33	1	16	76,152	------	------	------	Pre-development
2	Rational	170.93	1	11	112,816	------	------	------	Post-Development
3	Reservoir	8.288	1	21	105,284	2	354.96	109,197	Pond

Hydrograph Report

Hyd. No. 1

Pre-development

Hydrograph type	=	Rational	Peak discharge	=	79.33 cfs
Storm frequency	=	25 yrs	Time to peak	=	0.27 hrs
Time interval	=	1 min	Hyd. volume	=	76,152 cuft
Drainage area	=	28.910 ac	Runoff coeff.	=	0.47
Intensity	=	5.838 in/hr	Tc by User	=	16.00 min
IDF Curve	=	Bryant 50.IDF	Asc/Rec limb fact	=	1/1



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2024

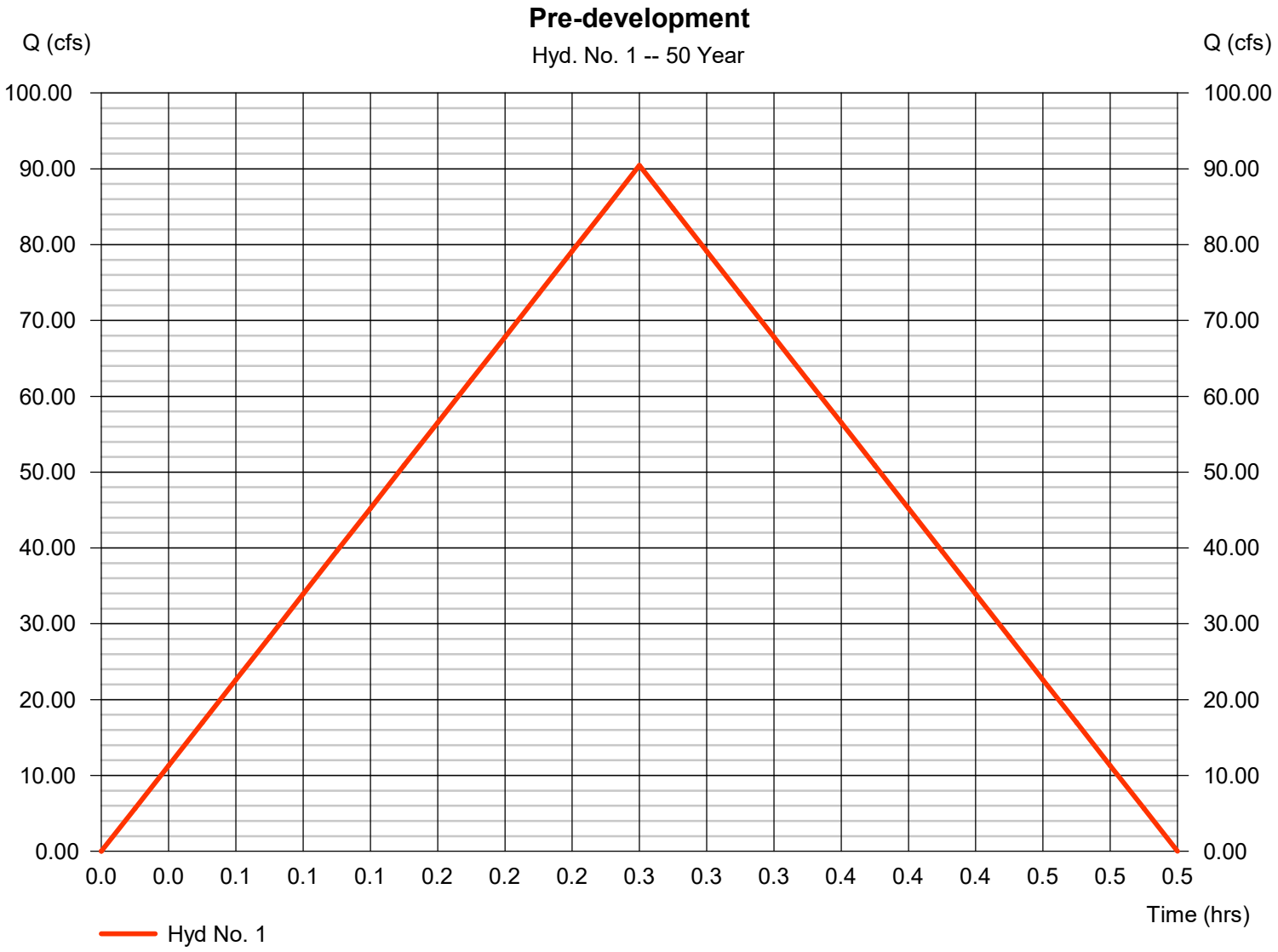
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	90.45	1	16	86,827	------	------	------	Pre-development
2	Rational	194.22	1	11	128,188	------	------	------	Post-Development
3	Reservoir	11.98	1	21	120,591	2	355.14	122,983	Pond

Hydrograph Report

Hyd. No. 1

Pre-development

Hydrograph type	=	Rational	Peak discharge	=	90.45 cfs
Storm frequency	=	50 yrs	Time to peak	=	0.27 hrs
Time interval	=	1 min	Hyd. volume	=	86,827 cuft
Drainage area	=	28.910 ac	Runoff coeff.	=	0.47
Intensity	=	6.656 in/hr	Tc by User	=	16.00 min
IDF Curve	=	Bryant 50.IDF	Asc/Rec limb fact	=	1/1



Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2024

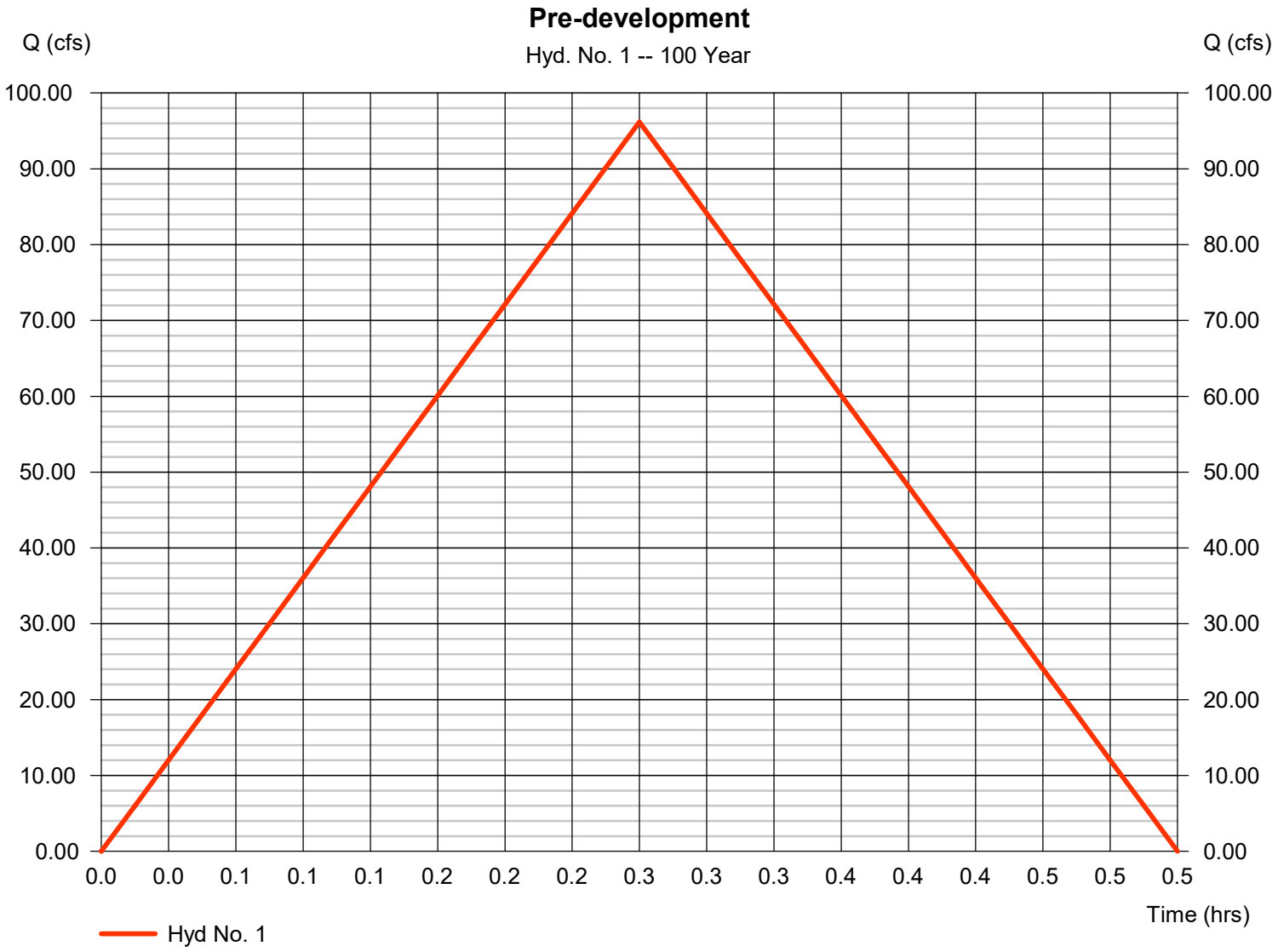
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	Rational	96.16	1	16	92,318	------	------	------	Pre-development
2	Rational	205.89	1	11	135,890	------	------	------	Post-Development
3	Reservoir	13.95	1	21	128,267	2	355.22	129,785	Pond

Hydrograph Report

Hyd. No. 1

Pre-development

Hydrograph type	=	Rational	Peak discharge	=	96.16 cfs
Storm frequency	=	100 yrs	Time to peak	=	0.27 hrs
Time interval	=	1 min	Hyd. volume	=	92,318 cuft
Drainage area	=	28.910 ac	Runoff coeff.	=	0.47
Intensity	=	7.077 in/hr	Tc by User	=	16.00 min
IDF Curve	=	Bryant 50.IDF	Asc/Rec limb fact	=	1/1



Multi-Hydrograph Plot

Hyd. No. 1

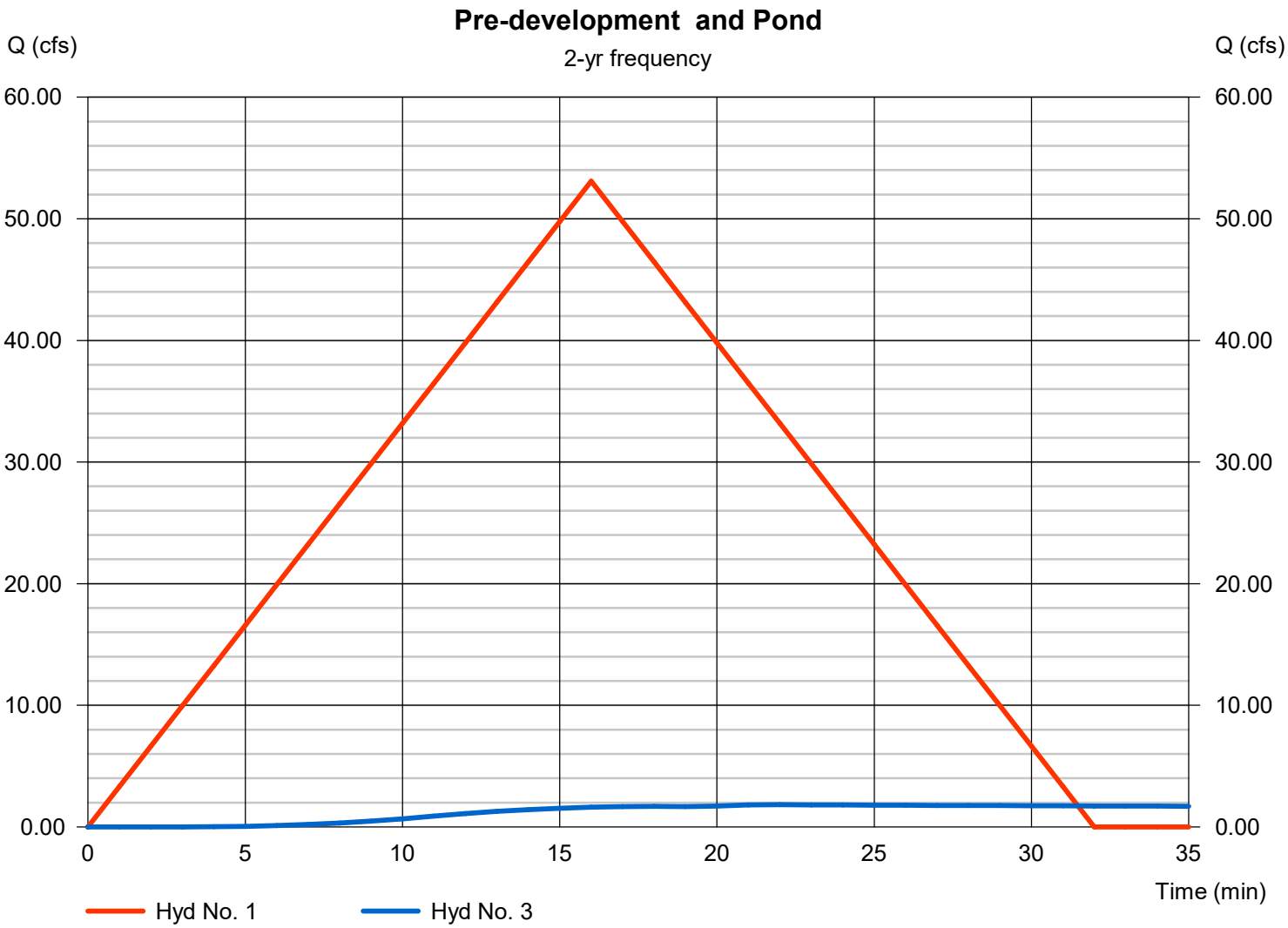
Pre-development

Hydrograph type	=	Rational
Peak discharge	=	53.08 cfs
Time to peak	=	16 min
Hyd. Volume	=	50,961 cuft

Hyd. No. 3

Pond

Hydrograph type	=	Reservoir
Peak discharge	=	1.83 cfs
Time to peak	=	22 min
Hyd. Volume	=	69,981 cuft



Multi-Hydrograph Plot

Hyd. No. 1

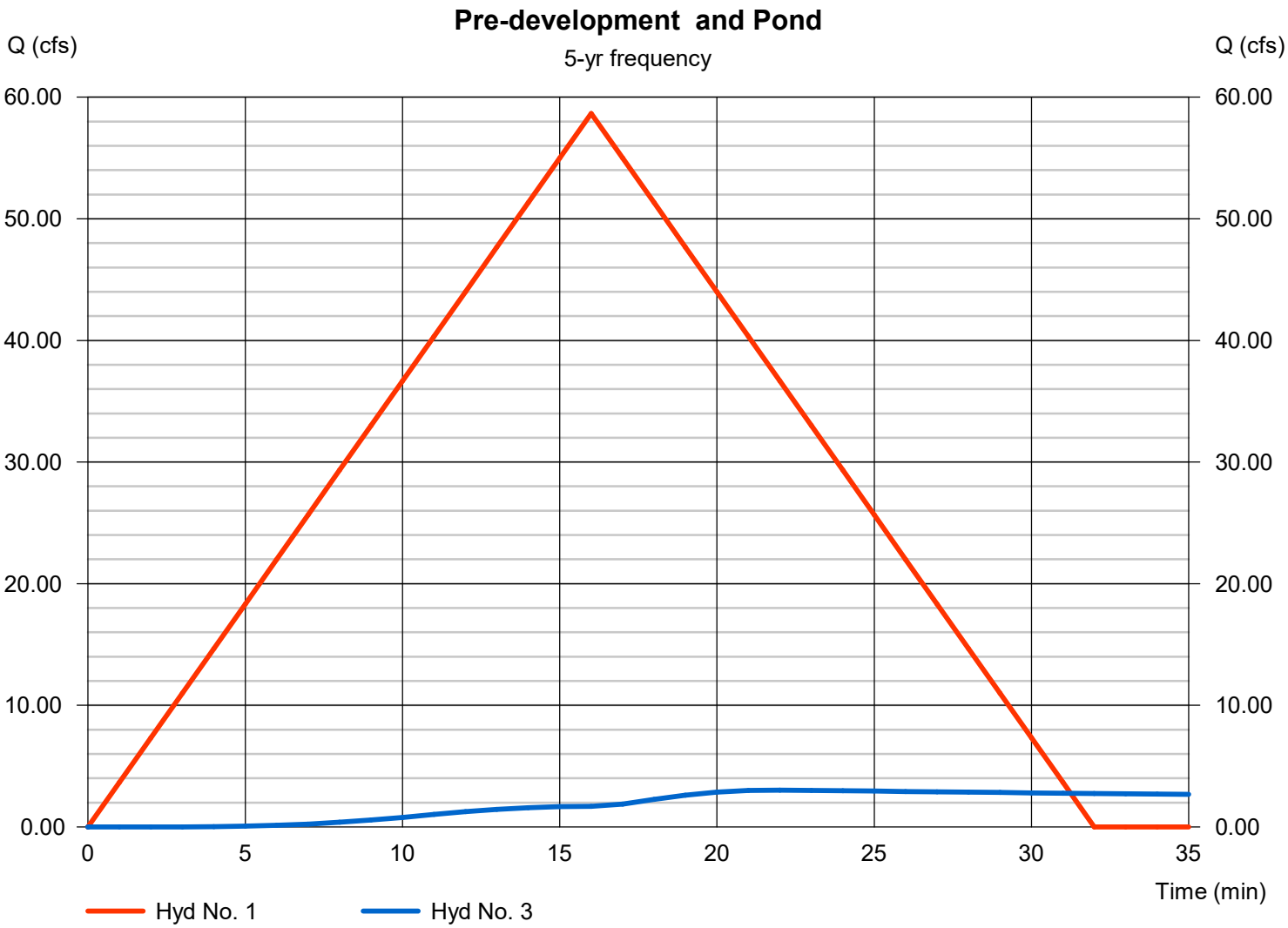
Pre-development

Hydrograph type	=	Rational
Peak discharge	=	58.66 cfs
Time to peak	=	16 min
Hyd. Volume	=	56,310 cuft

Hyd. No. 3

Pond

Hydrograph type	=	Reservoir
Peak discharge	=	3.03 cfs
Time to peak	=	22 min
Hyd. Volume	=	78,671 cuft



Multi-Hydrograph Plot

Hyd. No. 1

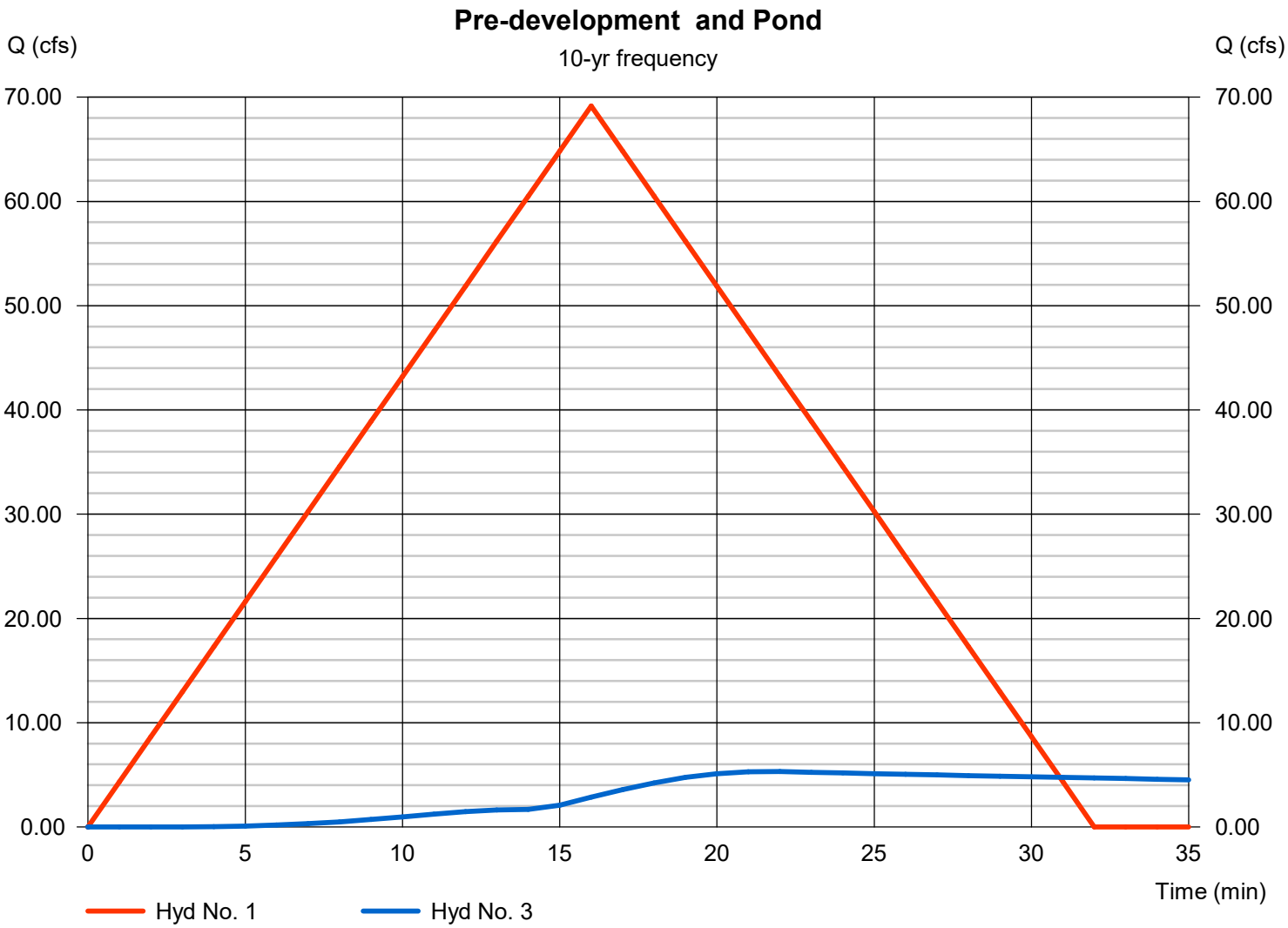
Pre-development

Hydrograph type	=	Rational
Peak discharge	=	69.15 cfs
Time to peak	=	16 min
Hyd. Volume	=	66,385 cuft

Hyd. No. 3

Pond

Hydrograph type	=	Reservoir
Peak discharge	=	5.32 cfs
Time to peak	=	22 min
Hyd. Volume	=	91,328 cuft



Multi-Hydrograph Plot

Hyd. No. 1

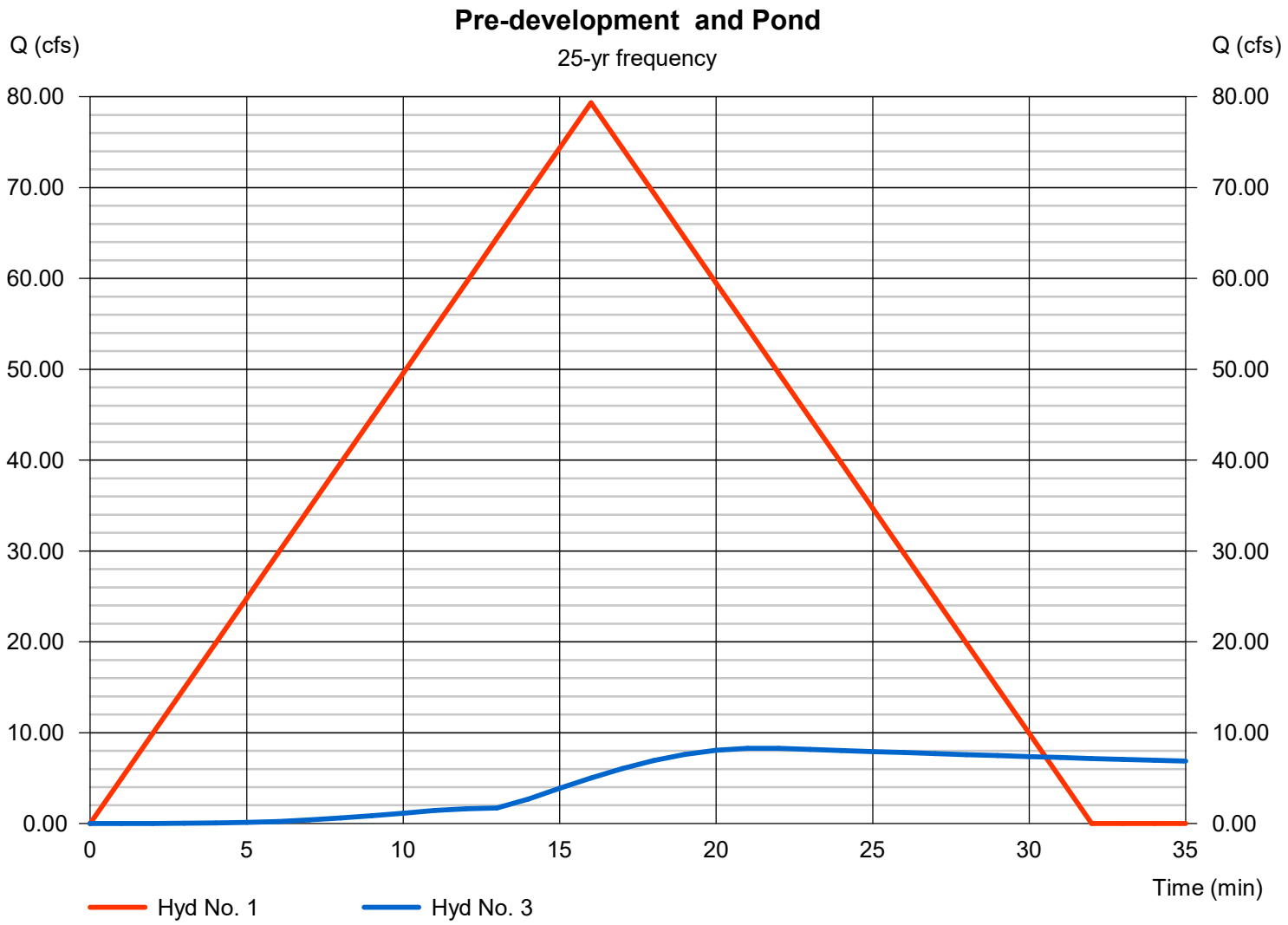
Pre-development

Hydrograph type	=	Rational
Peak discharge	=	79.33 cfs
Time to peak	=	16 min
Hyd. Volume	=	76,152 cuft

Hyd. No. 3

Pond

Hydrograph type	=	Reservoir
Peak discharge	=	8.29 cfs
Time to peak	=	21 min
Hyd. Volume	=	105,284 cuft



Multi-Hydrograph Plot

Hyd. No. 1

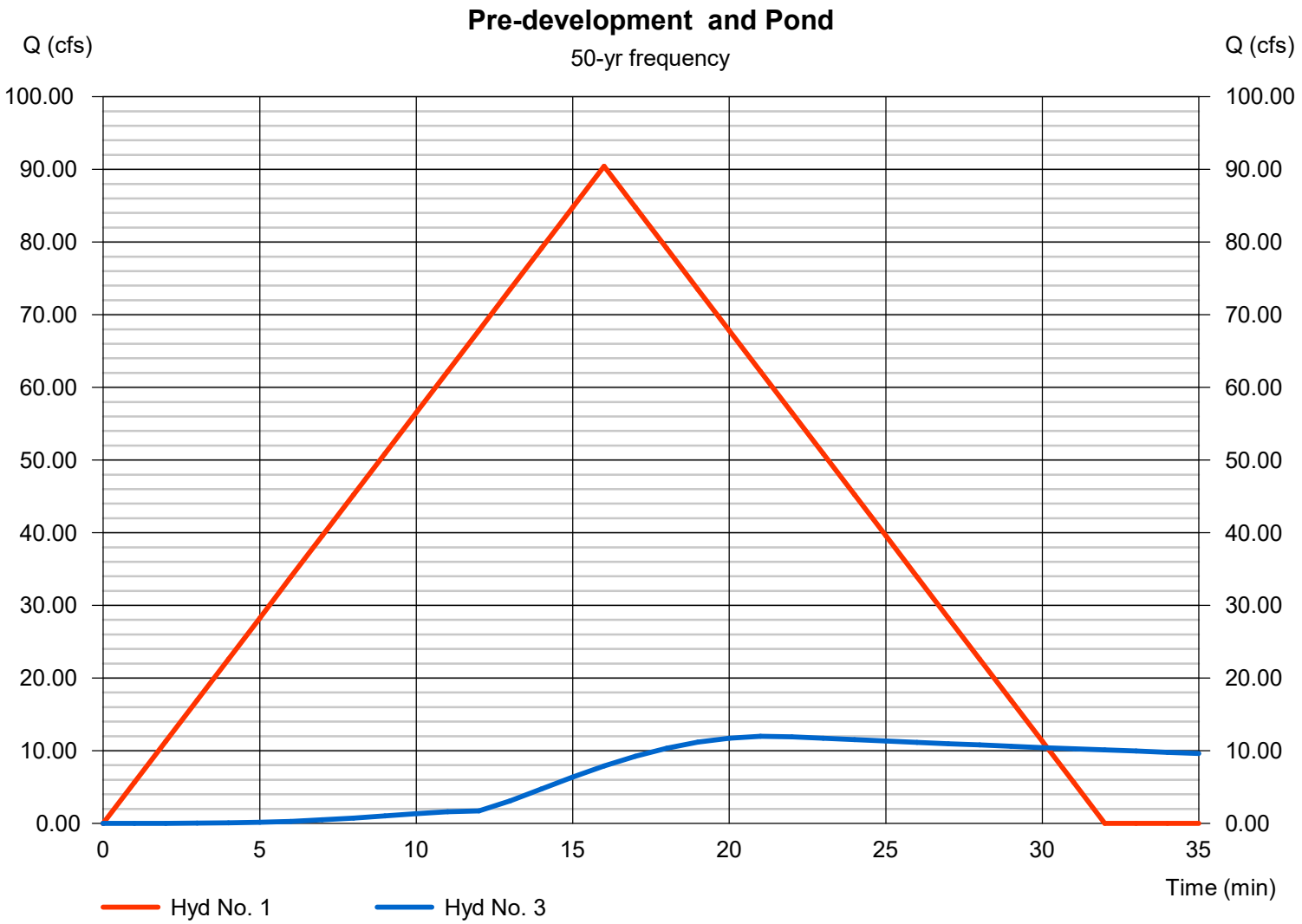
Pre-development

Hydrograph type	=	Rational
Peak discharge	=	90.45 cfs
Time to peak	=	16 min
Hyd. Volume	=	86,827 cuft

Hyd. No. 3

Pond

Hydrograph type	=	Reservoir
Peak discharge	=	11.98 cfs
Time to peak	=	21 min
Hyd. Volume	=	120,591 cuft



Multi-Hydrograph Plot

Hyd. No. 1

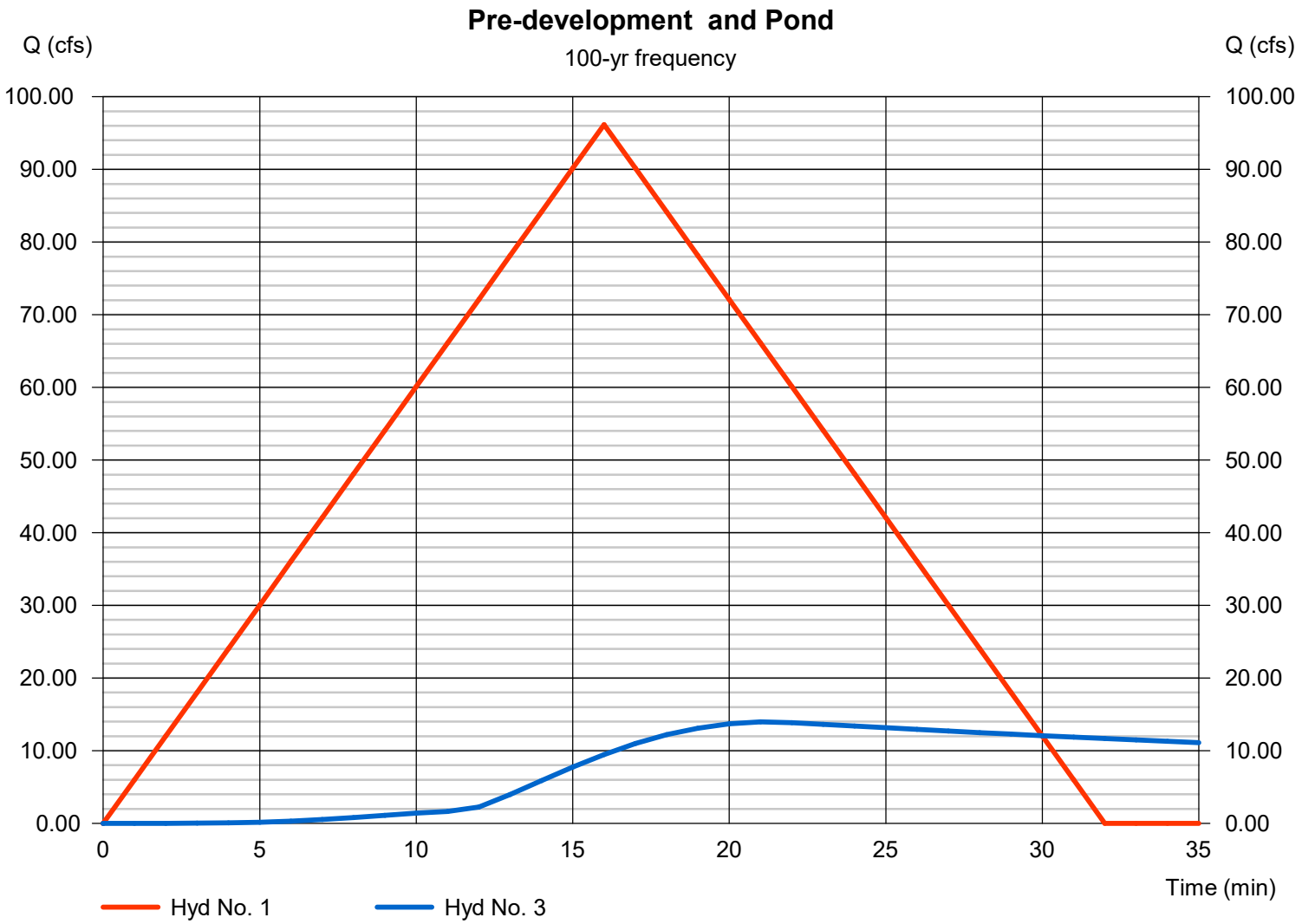
Pre-development

Hydrograph type	=	Rational
Peak discharge	=	96.16 cfs
Time to peak	=	16 min
Hyd. Volume	=	92,318 cuft

Hyd. No. 3

Pond

Hydrograph type	=	Reservoir
Peak discharge	=	13.95 cfs
Time to peak	=	21 min
Hyd. Volume	=	128,267 cuft



Pond Report

Pond No. 1 - <New Pond>

Pond Data

Trapezoid -Bottom L x W = 241.5 x 300.0 ft, Side slope = 3.00:1, Bottom elev. = 353.50 ft, Depth = 2.00 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	353.50	72,450	0	0
0.20	353.70	73,101	14,555	14,555
0.40	353.90	73,755	14,686	29,241
0.60	354.10	74,412	14,817	44,057
0.80	354.30	75,072	14,948	59,006
1.00	354.50	75,735	15,081	74,087
1.20	354.70	76,401	15,214	89,300
1.40	354.90	77,069	15,347	104,647
1.60	355.10	77,741	15,481	120,128
1.80	355.30	78,415	15,616	135,743
2.00	355.50	79,092	15,751	151,494

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 12.00	Inactive	Inactive	0.00
Span (in)	= 12.00	0.00	0.00	0.00
No. Barrels	= 1	1	0	0
Invert El. (ft)	= 353.50	0.00	0.00	0.00
Length (ft)	= 36.00	0.00	0.00	0.00
Slope (%)	= 0.50	0.00	0.00	n/a
N-Value	= .013	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	No	No	No

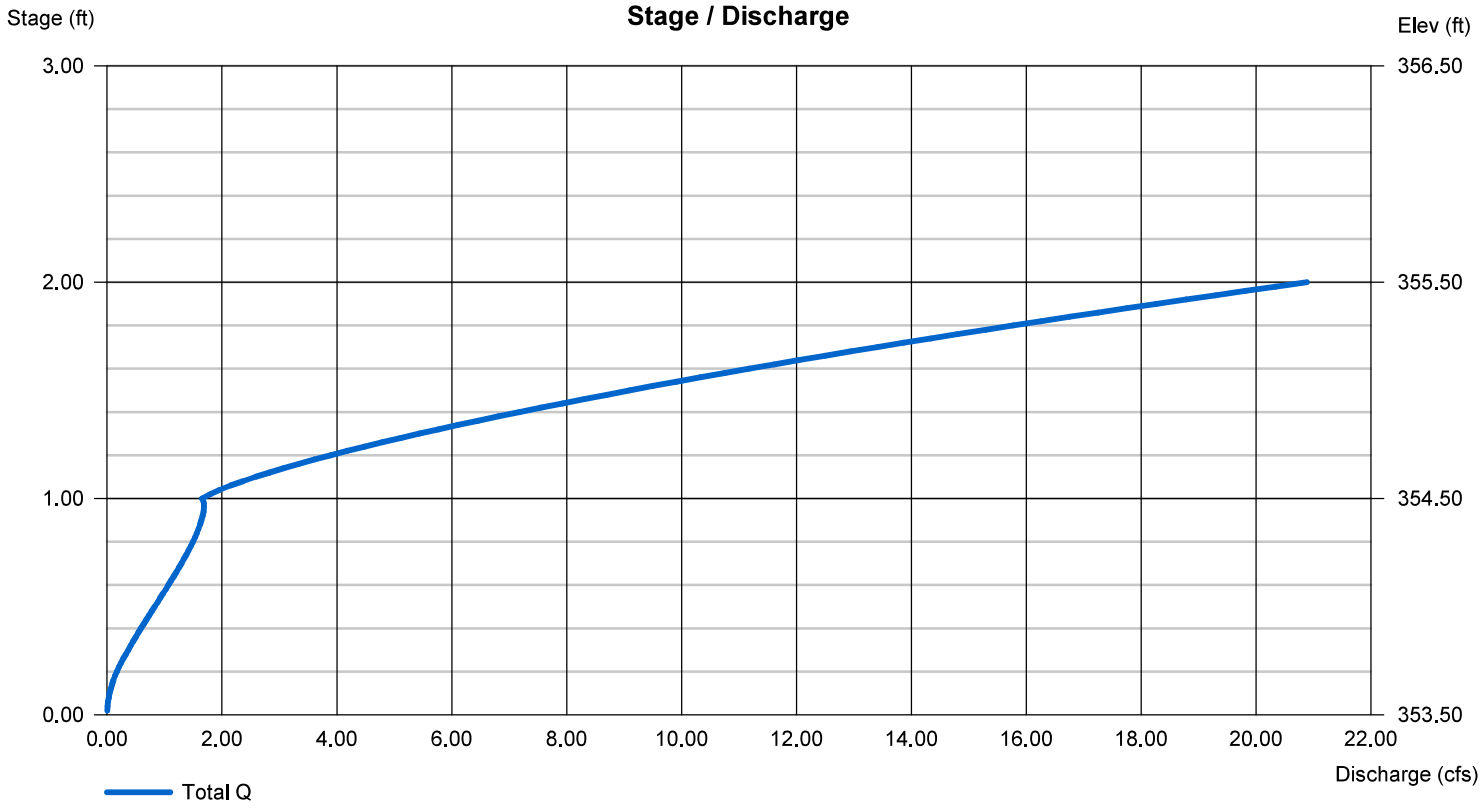
Weir Structures

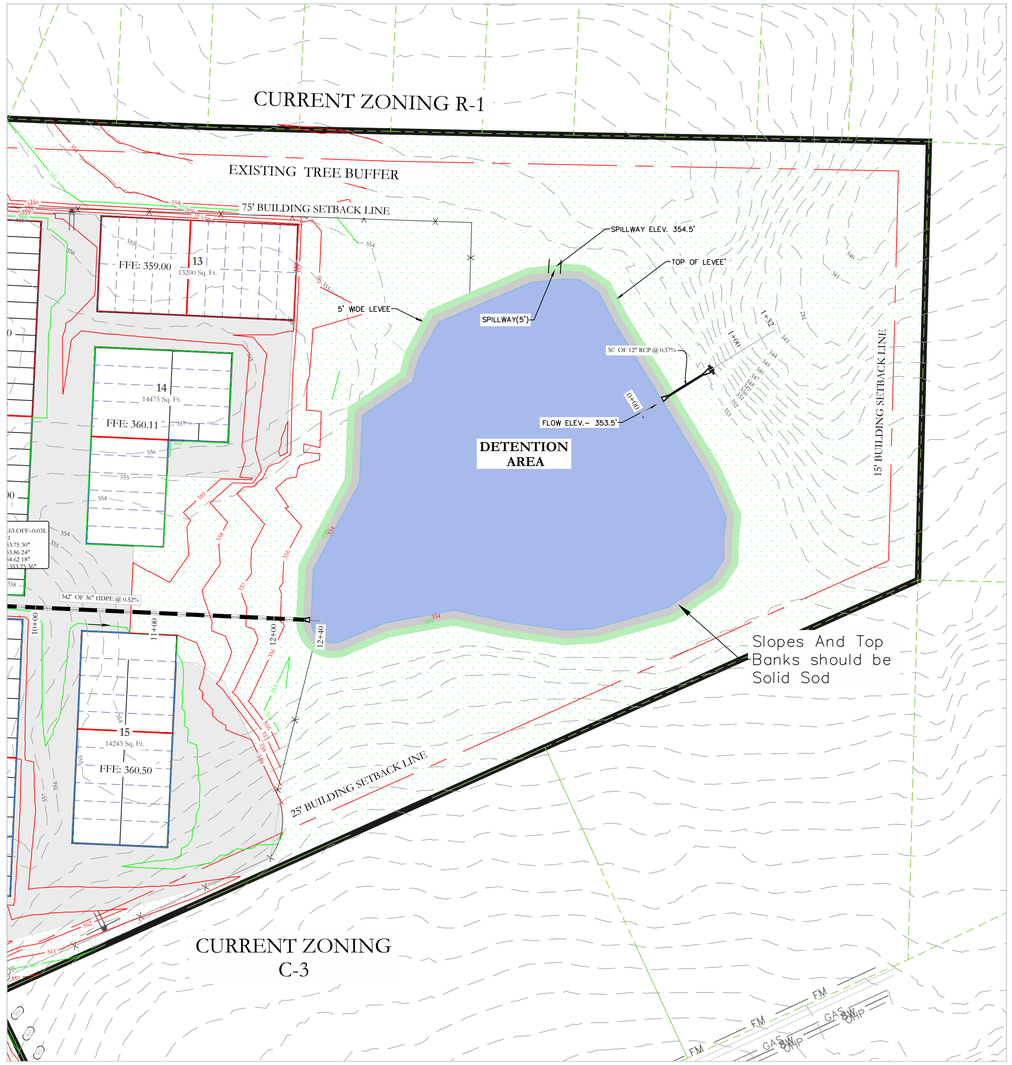
	[A]	[B]	[C]	[D]
Crest Len (ft)	= 5.00	0.00	0.00	0.00
Crest El. (ft)	= 354.50	0.00	0.00	0.00
Weir Coeff.	= 3.33	3.33	3.33	3.33
Weir Type	= Rect	---	---	---
Multi-Stage	= No	No	No	No

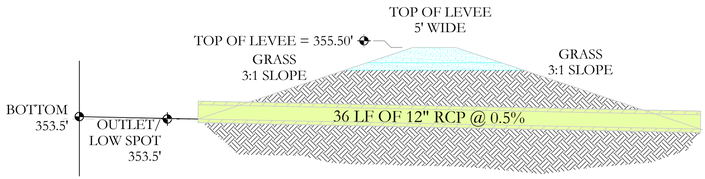
Exfil.(in/hr) = 0.000 (by Wet area)

TW Elev. (ft) = 0.00

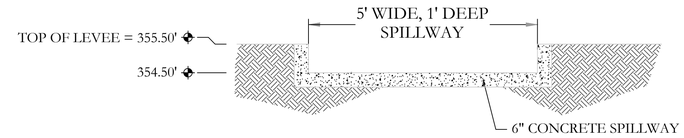
Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).







OUTLET SECTION
NTS



SPILLWAY END VIEW
NTS

LEGEND

EXISTING CONTOUR LINE 363
PROPOSED CONTOUR LINE 363
PROPOSED HDPE STORM PIPE
PROPOSED RCP STORM PIPE

DETENTION POND MAINTENANCE PLAN

Background
There will be one retention pond in this project.The retention pond is located at the North-East of the subject property. It is designed to temporarily detain stormwater to meet water quantity criteria before discharging off the property.

Routine Maintenance
The property owners association will maintain the drainage easements. Routine maintenance will include but not be limited to:
-Mowing of the bank slopes and area around the pond on a monthly basis during the growing season and as needed during the cooler months.

-The outlet pipe from the pond and other areas will be inspected monthly for debris which could inhibit the proper flow of discharge. Any debris will be removed immediately and disposed of or placed in a location to prevent future maintenance and to not cause impact up or downstream of the structure.

-Trash will be removed from around the pond to prevent entering the pond. Generally, the site should be kept free of loose trash which could be carried off site by wind or rain.

-Inspect the pond and outlet pipe for non-routine maintenance need.

Periodic or Non-Routine Maintenance
The routine inspection of the pond area and discharge pipe will identify needed repairs and non-routine maintenance. These items may include but not be limited to:

-Re-growth of trees on or around the pond bank. These should be cut and removed from the pond area.

-Sediment from the site may accumulate in the pond bottom and reduce the pond to below design volume requirements. The pond should be excavated if the pond bottom elevation reached a level that allows excessive aquatic growth or reduces the pond efficiency such, that the sediments are passing the discharge structure and release off site.

-Stabilization or re-grading of side slopes may be required periodically or after excessive rain events. Any disturbance of slopes should be reseeded or may require installation of erosion control materials until seeding can reestablish adequate grasses to prevent future erosion.

-Any other maintenance or repairs which would minimize other maintenance to the pond or outfall structures.

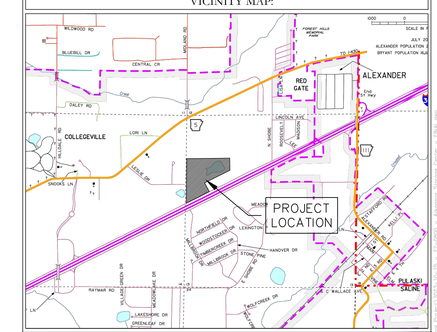
CERTIFICATE OF AUTHORIZATION
HOPE CONSULTING, INC.
No. 1931
ARKANSAS

N

BASIS OF BEARING:
GRID NORTH ARKANSAS
COORDINATE SYSTEM, SOUTH ZONE
BY GPS OBSERVATION

60' 30' 0 60'

VICINITY MAP:



HOPE CONSULTING
ENGINEERS - SURVEYORS
129 N. Main Street,
Benton, Arkansas 72015
PH. (501)315-2626
FAX (501) 315-0024
www.hopeconsulting.com

FOR USE AND BENEFIT OF:
STUART FINLEY

ARKANSAS STORAGE CENTER
RETENTION POND PLAN
BRYANT, SALINE COUNTY, ARKANSAS

DATE: 12-06-2023	C.A.D. BY:		DRAWING NUMBER:
REVISED:	CHECKED BY:		22-0800
SHEET: C-5.4	SCALE:		

500	01S	14W	0	21	300	62	1762

THESE PLANS ARE THE PROPERTY OF HOPE CONSULTING, INC. AND ARE NOT TO BE USED IN ANY WAY WITHOUT THE WRITTEN PERMISSION OF HOPE CONSULTING, INC.

CURRENT ZONING R-1

CURRENT ZONING C-2

EXISTING TREE BUFFER

75' BUILDING SETBACK LINE

25' BUILDING SETBACK LINE

15' BUILDING SETBACK LINE

DETENTION AREA

CURRENT ZONING C-3

CURRENT ZONING R-E

55' BUILDING SETBACK LINE

EXISTING TREE BUFFER

50' BUILDING SETBACK LINE

I-30 FRONTAGE ROAD

PRE-DEVELOPMENT WATER-FLOW

PRE-DEVELOPMENT CATCHING AREA

1 — FFE: 361.50 — 12300 Sq. Ft.
2 — FFE: 364.00 — 15900 Sq. Ft.
3 — FFE: 367.50 — 30060 Sq. Ft.
4 — FFE: 362.00 — FFE: 361.00 — 17440 Sq. Ft.
5 — FFE: 362.00 — FFE: 363.00 — 21200 Sq. Ft.
6 — FFE: 366.00 — FFE: 367.00 — 34160 Sq. Ft.
7 — FFE: 361.50 — FFE: 360.50 — 22960 Sq. Ft.
8 — FFE: 360.00 — FFE: 361.00 — 21200 Sq. Ft.
9 — FFE: 364.50 — FFE: 365.50 — 28160 Sq. Ft.
10 — FFE: 365.00 — FFE: 366.50 — 17000 Sq. Ft.
11 — FFE: 361.00 — FFE: 360.00 — 25120 Sq. Ft.
12 — FFE: 361.50 — FFE: 362.50 — 18500 Sq. Ft.
13 — FFE: 359.00 — 13240 Sq. Ft.
14 — FFE: 360.11 — 14475 Sq. Ft.
15 — FFE: 360.50 — 14243 Sq. Ft.

	Pre-Development	Post-Development without Detention	Post-Development with Detention
	Peak Flow (cfs)	Peak Flow (cfs)	Peak Flow (cfs)
2-Year	53.08	116.86	1.831
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10-Year	69.15	149.65	5.315
25-Year	79.33	176.93	8.288
50-Year	90.45	194.22	11.98
100-Year	96.16	205.89	13.95
TOC	**16.05 min**	**10.58 min**	

LEGEND

EXISTING CONTOUR LINE — 363

PROPOSED CONTOUR LINE — 363

PROPOSED HDPE STORM PIPE

PROPOSED RCP STORM PIPE

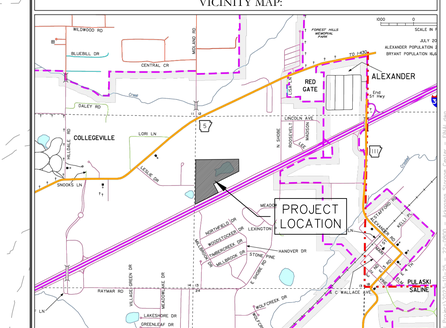
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ARKANSAS

N

BASIS OF BEARING
GRID NORTH, ARKANSAS
COORDINATE SYSTEM, SOUTH ZONE
BY GPS OBSERVATION

60' 30' 0 60'

VICINITY MAP:



Pre-Development Time of Concentration

Reach	Description of Flow	n	Length (ft)	First Elevation	Second Elevation	Δ Elevation (ft)	Slope (ft/ft)	Cross. Area (sqft)	Welted Perimeter (ft)	Hydraulic Radius	Velocity (Ft/s)	Travel Time (min)
A-B	Overland	0.2	40.00	377.50	375.40	2.10	0.05					9.57
B-C	Shallow Flow	0.03	410.00	375.40	366.52	8.88	0.02				2.37	2.88
D-E	Open Channel	0.03	220.00	366.52	361.22	5.30	0.02	3.75	10.10	0.37	3.97	0.92
E-F	Open Channel	0.03	200.00	361.22	355.50	5.72	0.03	3.75	10.10	0.37	4.32	0.77
F-G	Open Channel	0.03	258.05	355.50	353.50	2.00	0.01	3.75	10.10	0.37	2.25	1.91
											Total Time	**16.05**

HOPE CONSULTING
ENGINEERS - SURVEYORS

129 N. Main Street,
Benton, Arkansas 72015
PH. (501)315-2626
FAX (501) 315-0024
www.hopeconsulting.com

FOR USE AND BENEFIT OF:
STUART FINLEY

ARKANSAS STORAGE CENTER
PRE-DEVELOPMENT FLOW
BRYANT, SALINE COUNTY, ARKANSAS

DATE:	12-06-2023	C.A.D. BY:	DRAWING NUMBER:
REVISED:		CHECKED BY:	22-0800
SHEET:	C-5.5	SCALE: 1" = 60'	

500	01S	14W	0	21	300	62	1762

CURRENT ZONING R-1

CURRENT ZONING C-2

CURRENT ZONING R-E

CURRENT ZONING C-3

EXISTING TREE BUFFER

25' BUILDING SETBACK LINE

75' BUILDING SETBACK LINE

15' BUILDING SETBACK LINE

55' BUILDING SETBACK LINE

50' BUILDING SETBACK LINE

EXISTING TREE BUFFER

I-30 FRONTAGE ROAD

5' WIDE LEVEE

SPILLWAY ELEV. 355.5'

TOP OF LEVEE

SPILLWAY(5')

FLOW ELEV.= 353.5'

DETENTION AREA

POST-DEVELOPMENT WATER-FLOW

POST-DEVELOPMENT WATER-FLOW

POST-DEVELOPMENT WATER-FLOW

POST-DEVELOPMENT CATCHING AREA

1 — 12500 Sq. Ft. — FFE: 361.50
2 — 15900 Sq. Ft. — FFE: 364.00
3 — 30060 Sq. Ft. — FFE: 367.50
4 — 17440 Sq. Ft. — FFE: 362.00 / FFE: 361.00
5 — 21200 Sq. Ft. — FFE: 362.00 / FFE: 363.00
6 — 34160 Sq. Ft. — FFE: 366.00 / FFE: 367.00 / FFE: 368.00
7 — 22060 Sq. Ft. — FFE: 361.50 / FFE: 360.50
8 — 21200 Sq. Ft. — FFE: 360.00 / FFE: 361.00
9 — 28160 Sq. Ft. — FFE: 364.50 / FFE: 365.50 / FFE: 366.50
10 — 17000 Sq. Ft. — FFE: 365.00
11 — 25120 Sq. Ft. — FFE: 361.00 / FFE: 360.00
12 — 18560 Sq. Ft. — FFE: 361.50 / FFE: 362.50
13 — 13200 Sq. Ft. — FFE: 359.00
14 — 14473 Sq. Ft. — FFE: 360.11
15 — 14243 Sq. Ft. — FFE: 360.50

LEGEND

EXISTING CONTOUR LINE 363

PROPOSED CONTOUR LINE 363

PROPOSED HDPE STORM PIPE

PROPOSED RCP STORM PIPE

	Pre-Development Peak Flow (cfs)	Post-Development without Detention Peak Flow (cfs)	Post-Development with Detention Peak Flow (cfs)
2-Year	53.08	116.86	2.028
5-Year	58.66	130.27	2.263
10-Year	69.15	149.65	2.617
25-Year	79.33	176.93	2.955
50-Year	90.45	194.22	3.283
100-Year	96.16	205.89	3.435
TOC	**16.05 min**	**10.58 min**	

Post-Development Time of Concentration

Reach	Description of Flow	n	Length (ft)	First Elevation	Second Elevation	Δ Elevation (ft)	Slope (ft/ft)	Cross. Area (sqft)	Diameter (ft)	Welted Perimeter (ft)	Hydraulic Radius	Velocity (Ft/s)	Travel Time (min)
A-B	Overland	0.02	40.00				0.020						5.29
B-C	Shallow Flow		234.00	375.82	372.04	3.78	0.020					2.87	1.36
C-D	18" Pipe	0.015	302.00				0.005		1.50		0.38	3.64	1.38
D-E	18" Pipe	0.015	145.00				0.010		2.00		0.50	6.34	0.38
E-F	24" Pipe	0.015	140.00				0.012		2.00		0.50	6.70	0.35
F-G	24" Pipe	0.015	296.00				0.007		2.50		0.63	6.19	0.80
G-H	36" Pipe	0.015	357.00				0.005		3.00		0.75	5.79	1.03
												Total Time	**10.58**

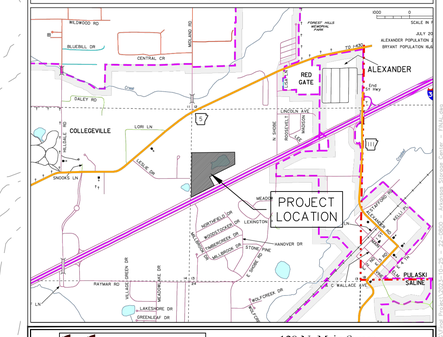
CERTIFICATE OF AUTHORIZATION
HOPE CONSULTING, INC.
No. 1931
ARKANSAS

BASIS OF BEARING:
GRID NORTH, ARKANSAS
COORDINATE SYSTEM, SOUTH ZONE
BY GPS OBSERVATION

N

60' 30' 0 60'

VICINITY MAP:



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